

ERRATA

Dear Customer:

Recently, we were made aware of some technical revisions that need to be applied to the *AASHTO LRFD Bridge Design Specifications*, Ninth Edition. Please scroll down to see the full erratum.

In the event that you need to download this file again, please download from AASHTO's online bookstore at:

<https://downloads.transportation.org/LRFBDS-9-Errata.pdf>

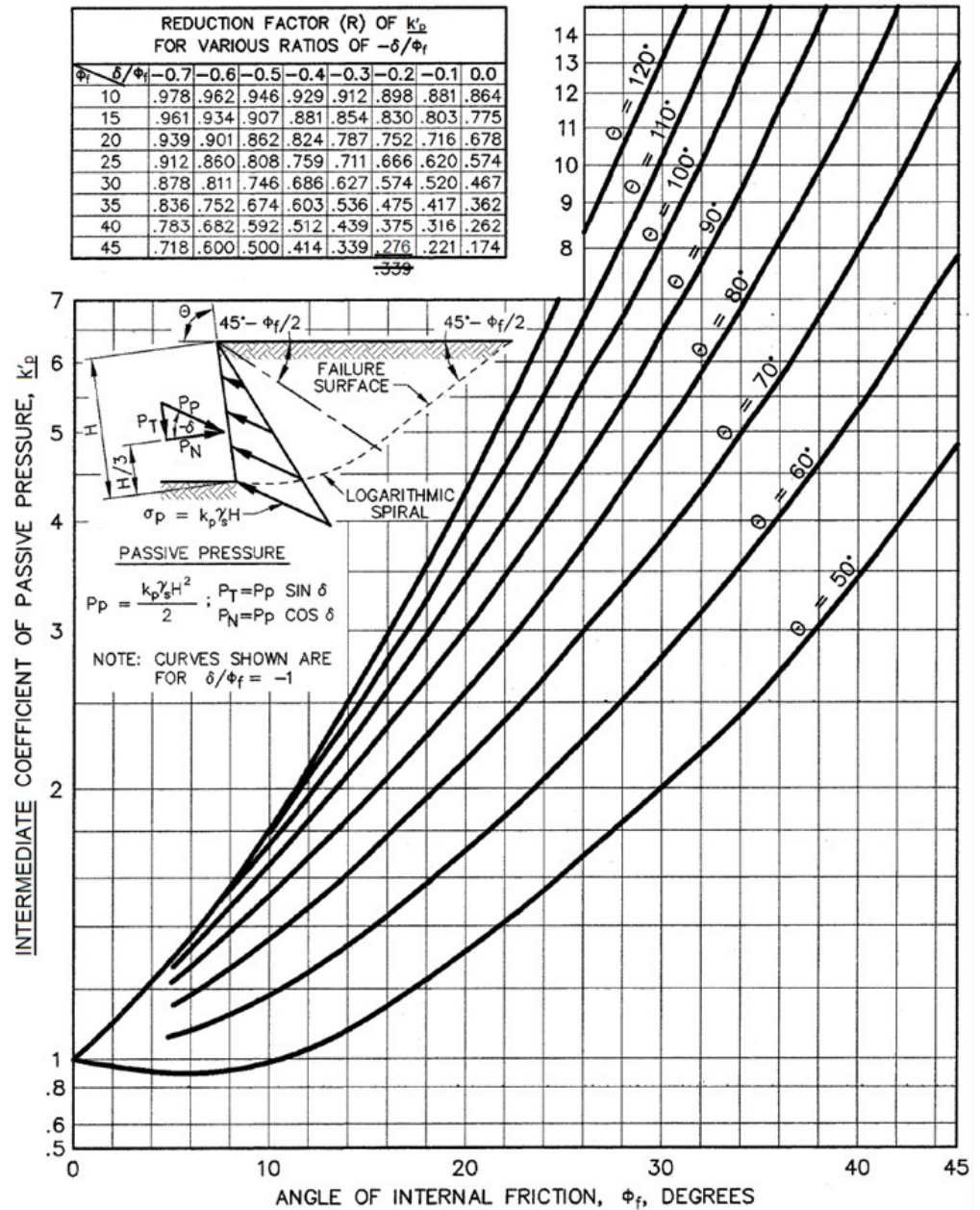
Then, please replace the existing pages with the corrected pages to ensure that your edition is both accurate and current.

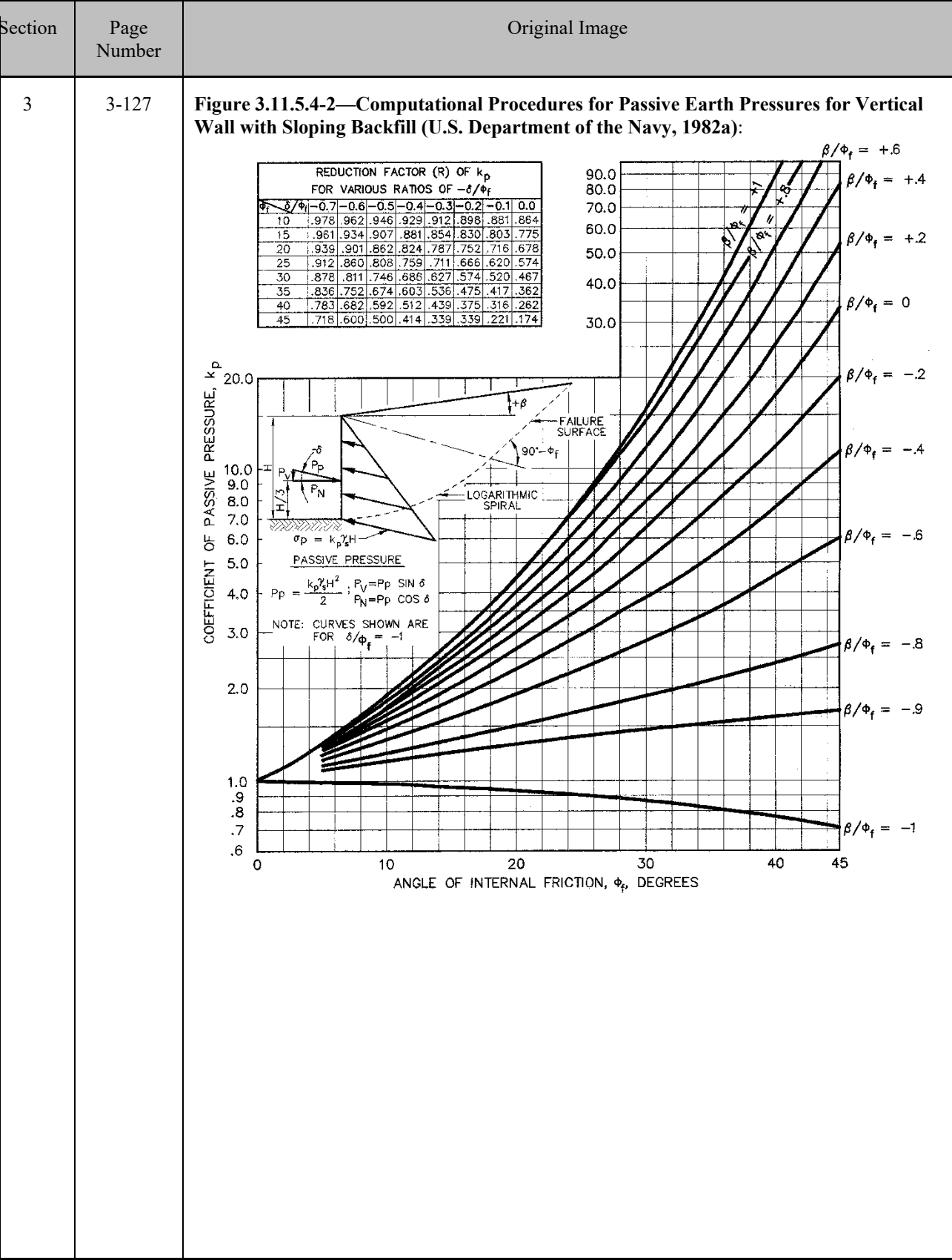
AASHTO staff sincerely apologizes for any inconvenience to our readers.

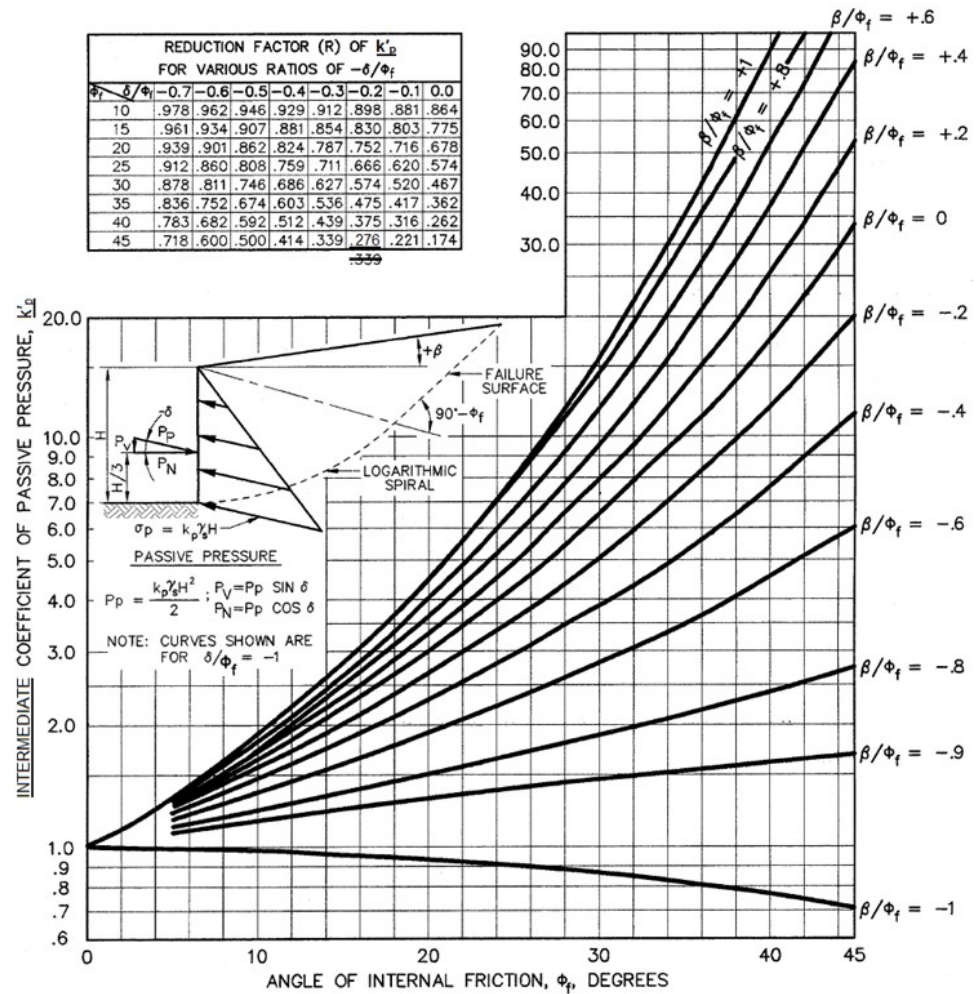
AASHTO Publications Staff
November 2021

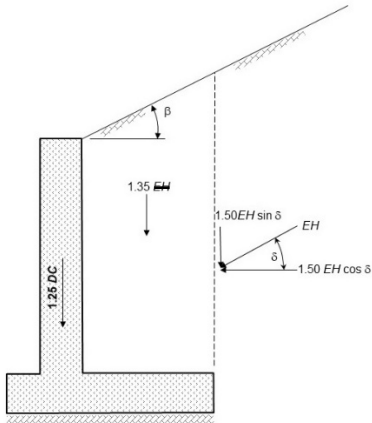
Summary of Errata for LRFDBDS-9, November 2021

Section	Page Number(s)	Original Image																																																																																										
3	3-126	Figure 3.11.5.4-1—Computational Procedures for Passive Earth Pressures for Vertical and Sloping Walls with Horizontal Backfill (U.S. Department of the Navy, 1982a): <table><tr><th colspan="9">REDUCTION FACTOR (R) OF k_p FOR VARIOUS RATIOS OF $-\delta/\phi_f$</th></tr><tr><th>δ/ϕ_f</th><th>-0.7</th><th>-0.6</th><th>-0.5</th><th>-0.4</th><th>-0.3</th><th>-0.2</th><th>-0.1</th><th>0.0</th></tr><tr><td>10</td><td>.978</td><td>.962</td><td>.946</td><td>.929</td><td>.912</td><td>.898</td><td>.881</td><td>.864</td></tr><tr><td>15</td><td>.961</td><td>.934</td><td>.907</td><td>.881</td><td>.854</td><td>.830</td><td>.803</td><td>.775</td></tr><tr><td>20</td><td>.939</td><td>.901</td><td>.862</td><td>.824</td><td>.787</td><td>.752</td><td>.716</td><td>.678</td></tr><tr><td>25</td><td>.912</td><td>.860</td><td>.808</td><td>.759</td><td>.711</td><td>.666</td><td>.620</td><td>.574</td></tr><tr><td>30</td><td>.878</td><td>.811</td><td>.746</td><td>.686</td><td>.627</td><td>.574</td><td>.520</td><td>.467</td></tr><tr><td>35</td><td>.836</td><td>.752</td><td>.674</td><td>.603</td><td>.536</td><td>.475</td><td>.417</td><td>.362</td></tr><tr><td>40</td><td>.783</td><td>.682</td><td>.592</td><td>.512</td><td>.439</td><td>.375</td><td>.316</td><td>.262</td></tr><tr><td>45</td><td>.718</td><td>.600</td><td>.500</td><td>.414</td><td>.339</td><td>.271</td><td>.211</td><td>.174</td></tr></table> COEFFICIENT OF PASSIVE PRESSURE, k_p ANGLE OF INTERNAL FRICTION, ϕ_f, DEGREES NOTE: CURVES SHOWN ARE FOR $\delta/\phi_f = -1$ PASSIVE PRESSURE $P_p = \frac{k_p \gamma H^2}{2}$; $P_T = P_p \sin \delta$ $P_N = P_p \cos \delta$ $\sigma_p = k_p \gamma H$ FAILURE SURFACE LOGARITHMIC SPIRAL	REDUCTION FACTOR (R) OF k_p FOR VARIOUS RATIOS OF $-\delta/\phi_f$									δ/ϕ_f	-0.7	-0.6	-0.5	-0.4	-0.3	-0.2	-0.1	0.0	10	.978	.962	.946	.929	.912	.898	.881	.864	15	.961	.934	.907	.881	.854	.830	.803	.775	20	.939	.901	.862	.824	.787	.752	.716	.678	25	.912	.860	.808	.759	.711	.666	.620	.574	30	.878	.811	.746	.686	.627	.574	.520	.467	35	.836	.752	.674	.603	.536	.475	.417	.362	40	.783	.682	.592	.512	.439	.375	.316	.262	45	.718	.600	.500	.414	.339	.271	.211	.174
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Section	Page Number	Original Text	Corrected Text
6	6-308–309	First paragraph of 6.13.6.1.3c—Web Splices: “As a minimum, web splice plates and their connections shall be designed at the strength limit state for a design web force taken equal to the smaller factored shear resistance of the web splice plates, $V_r = \phi_v V_n$, on either side of the splice determined according to the provisions of Article 6.10.9 or 6.11.9, as applicable.” Second paragraph of 6.13.6.1.3c—Web Splices: “Should the moment resistance provided by the flanges at the point of splice, determined as specified in Article 6.13.6.1.3b, not be sufficient to resist the factored moment at the strength limit state, the web splice plates and their connections shall instead be designed for a design web force taken equal to the vector sum of the smaller factored shear resistance and a horizontal force in the web that provides the necessary moment resistance in conjunction with the flanges.”	First paragraph of 6.13.6.1.3c—Web Splices: “As a minimum, web splice plates and their connections shall be designed at the strength limit state for a design web force taken equal to the smaller factored shear resistance of the web, $V_r = \phi_v V_n$, on either side of the splice determined according to the provisions of Article 6.10.9 or 6.11.9, as applicable.” Second paragraph of 6.13.6.1.3c—Web Splices: “Should the moment resistance provided by the flanges at the point of splice, determined as specified in Article 6.13.6.1.3b, not be sufficient to resist the factored moment at the strength limit state, the web splice connections shall instead be designed for a design web force taken equal to the vector sum of the smaller factored shear resistance and a horizontal force in the web that provides the necessary moment resistance in conjunction with the flanges.” Note: This change applies only to the PDF edition, and brings it in line with the print edition.
Section	Page Number	Original Image	
11	11-13	Figure C11.5.6-1—Typical Application of Load Factors for Bearing Resistance 	

Corrected Image		
Section	Page Number	Original Image
11	11-13	Figure C11.5.6-2—Typical Application of Load Factors for Sliding and Eccentricity

Corrected Image		

Section	Page Number	Original Text	Corrected Text
12	12-22		Table 12.6.6.3-1—Minimum Cover: The note in the table has been moved to eliminate potential confusion regarding concrete pipe measurements.
12	12-95	12.14.5.7—Crack Control: “The provisions of Article 5.6.4 for buried structures shall apply.”	12.14.5.7—Crack Control: “The provisions of Article 5.6.7 for buried structures shall apply.”