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ERRATA as of September 9, 2010
ACI 343R-95
Analysis and Design of Reinforced Concrete Bridge Structures
Reported by ACI Committee 343

5.5.6, page 58, formula for SF revised as:

$$SF = Kv^2 (0.515 Kv^2)$$

$$\underline{SF = KV^2 (0.515 KV^2)}$$

7.3.4.3, page 83 revised as:

For rectangular sections, the design moment strength can be computed as follows (ACI 318R)

$$M_n = A_s f_y d (1 - 0.59 \rho f_y / f'_c)$$

7.3.7, page 87, Eq. (7-45) revised as:

$$V_u \leq \phi N_n \quad (7-45)$$

$$\underline{V_u \leq \phi V_n \quad (7-45)}$$

Page 98, heading of Table 8 revised as:

Table 8.5.2—Recommended minimum thickness for constant depth members*

Page 103, Table 9.6.3.1 revised as:

Table 9.6.3.1—Friction coefficients for post-tensioning tendons*

Types of tendons and sheathing	Wobble coefficient, K		Curvature coefficient, H_u
	per ft	per m	

9.6.4.3, page 105 revised as:

...at the time of prestress, losses calculated from Eqs. (9-5) and ~~(9-6)~~ **(9-7)** should...

Page 106, Eq. (9-12) revised as:

$$V_u \leq V_{nh} \phi$$

10.5.3, page 120 revised as:

The live load bending moment for each section should be determined by applying to the beam the fraction of a wheel load (both front and rear), determined by the following relations

$$DF = [(12n_e + 9)/\cancel{nb}n_b]/(5 + n_e/10) + [(3 - 2n_e/7) (1 - C/3)2]$$

or when C is 3 or greater, use

$$DF = [(12n_e + 9)/\cancel{nb}n_b]/(5 + n_e/10)$$

Fig. 11.3.6.1, page 129 Stage 1(b) caption revised as:

b. Build Lower Part of Caisson with Cutting Edge (~~Shee~~) (**Sheet**)

11.6.4.3, page 136 revised as:

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- 1.5 Footing anchored on rock
- 3.0 Footing not anchored on rock
- 5.0 Footing on soil
- 1.0 Footing on multiple rows of piles

11.6.5, page 138 revised as:

...followed by casting the pier cap [Fig. 11.6.5(~~ed~~) and (~~de~~)].

Last revised, 7-10-2010, 3:25 p.m.