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ERRATA
ACI-IPS-1
Essential Requirements for R/C Buildings
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Eq. (1-3), page 8, revised as:

$$\text{Design Strength} \geq \text{Required Factored Strength} \quad (1-3)$$

Chapter 2, page 16, revised as:

Required ~~factored~~ strength — Strength of a member or cross section required to resist factored loads or related internal moments and forces in specified load combinations.

→ Insert line before definition of *Retaining wall*

Page 18:

Delete line after definition of A_i .

Page 22 revised as:

V = basic wind speed, mph, corresponding to 3 s gust speed at 33 ft above ground in exposure Category C

Page 41, commentary revised as:

“Table 4.13 gives typical values of basic wind speed given as a guide for regions outside of the United States and ~~other~~ countries where wind speed velocity information is not available.”

Section 4.11.2.2, page 43, revised as:

4.11.2.2 — Effective peak ground acceleration. This ground motion representation shall be described through an effective peak ground horizontal acceleration in rock for short periods of vibration, A_a , expressed as a fraction of gravity, g , exceeds the value from Fig. 4.1, the design shall not be preformed using the *Essential Requirements*, and the supporting codes and standards listed in 1.4 shall be used.

. The values given in the general building code having jurisdiction shall be used for obtaining the corresponding values. See 1.4.

Section 7.3.8, page 81, revised as:

7.3.8 — Corner reinforcement

Corner top and bottom slab reinforcement, in addition to other reinforcement, shall be provided at exterior supported corners of the slab (for cantilevers see 7.6) for a distance equal to one-fifth of the longer clear span of the slab panel (Fig. 7.4). The area of corner reinforcement, top and bottom, shall be sufficient to resist a moment equal to the required positive flexural strength, per unit of width, in the slab panel, in accordance with 7.3.8.1 and 7.3.8.2.

Section 8.4.3.3, page 106, revised as:

8.4.3.3 — Joists. In joists with web widths, b_w , less than or equal to 6 in., either one longitudinal bar or a two-bar bundle in contact, located one above the other, is permitted. For web widths greater than 6 in. and less than 8 in., either one longitudinal bar or two bars in a single layer are permitted, but ~~multiple layers are~~ bundling of reinforcement is not permitted. For web widths equal to or greater than 8 in., the maximum number of bars in a single layer shall be one more than those allowed for in Table 8.3.

Section 8.4.11.2, page 112, revised as:

8.4.11.2 - Transverse flange reinforcement. In the slab, reinforcement perpendicular to the beam shall resist the factored required negative moment computed by assuming the flange width acts as a cantilever supported by the beam (Fig. 8.12); and for isolated T-beams the full width of overhanging flange. This reinforcement shall comply with the requirements for negative moment reinforcement in slabs as set forth in 7.3.6.

Section 9.4.1 (c), page 132, revised as:

- (c) ~~;~~ The slab reinforcement in waffle-slabs over cellular spaces shall comply with the requirements of 7.5;

Section 9.4.3.2, page 133, revised as:

9.4.3.2 - Location. Negative moment flexural reinforcement shall be provided in both directions at supports perpendicular to edge and intermediate supporting girders, beams, and reinforced concrete walls. Negative moment flexural reinforcement shall be located as close as concrete cover requirements permit (5.4.1) to the upper surface of the slab. The short span negative moment flexural reinforcement shall be located above the long span negative moment flexural reinforcement.

Section 10.5.3, page 157, revised as:

These verifications shall be performed at the upper and lower sections of the column of the same story.

Section 12.6.3(a), page 189, revised as:

~~Eq. (7-23)~~ **Eq. (7-4)**

Section 14.5.4.1, page 203:

the reference at the end should be 14.5.4.

Section 14.5.6.2, page 206, revised as: