



American Concrete Institute®
Advancing concrete knowledge

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Page 318, Footnote to Table A.7, Design Aid, revised as:

λ = lightweight-aggregate concrete factor (~~1.3~~ 0.75 for lightweight concrete, and 1.0 for normalweight concrete).

Page 404, Step 7, 11.6.2, Column 3, equation revised as:

$$\rho_l \geq 0.0025 + 0.5(2.5 - 90 \text{ ft} / 28 \text{ ft})$$

$$\times \left(\frac{0.62 \text{ in.}^2}{(12 \text{ in.})(12 \text{ in.})} - 0.0025 \right)$$

$$\rho_l \geq 0.0019$$

$$\rho_l \geq 0.0025 + 0.5(2.5 - 92 \text{ ft} / 28 \text{ ft})$$
$$\times \left(\frac{0.62 \text{ in.}^2}{(12 \text{ in.})(12 \text{ in.})} - 0.0025 \right)$$

$$\rho_l \geq 0.0018$$

Page 404, Step 8, 11.7.4.1, Column 3, revised as:

The maximum factored axial load is 1015 kips or 1,015,000 lb and the maximum factored moment is ~~3718, 6300~~ ft-kip or ~~446,400,000~~223,200,000 in.-lb. The factored axial stress on the concrete due to the combined loads is:

$$\sigma = 1,015,000 \text{ lb} / \{ (12 \text{ in.})(28 \text{ ft})(12 \text{ in./ft}) \} + \del{446,400,000}\u{223,200,000} \text{ in.-lb} * 336 \text{ in.} / 37,933,056 \text{ in.}^4 = \del{4206}\u{2229} \text{ psi.}$$

Page 406, title, revised as:

Shear Wall Example 1 should read Shear Wall Example 2

Page 412, Step 5, 18.10.2.2, Column 3, revised as:

$$h_w/\ell_w \geq (9\textcolor{red}{2}6 \text{ ft})/(28 \text{ ft}) = 3.\textcolor{red}{3}4 > 2$$

Page 414, Step 5, 18.10.6.2, Column 3, revised as:

$$h_w/\ell_w = (9\textcolor{red}{2}6 \text{ ft})/(28 \text{ ft}) = 3.\textcolor{red}{3}4 \text{ ft} \quad \text{OK}$$

Page 414, Step 5, 18.10.6.2, Column 3, revised as:

~~$$\delta_u / h_w = \frac{(2.4 \text{ in.})(5)}{(96 \text{ ft})(12 \text{ in./ft})}$$~~

$$\delta_u / h_w = 0.0104$$

$$\delta_u / h_w = \frac{(2.4 \text{ in.})(5)}{(92 \text{ ft})(12 \text{ in./ft})}$$

$$\delta_u / h_w = 0.0109$$

Therefore, the value determined from Eq. (18.10.6.2) is:

~~$$c \geq \frac{336 \text{ in.}}{600(1.5*0.0104)} = 36 \text{ in.}$$~~

$$c \geq \frac{336 \text{ in.}}{600(1.5*0.0109)} = 34.25 \text{ in.}$$

Because c of 67.85 in. is greater than $\textcolor{red}{34.25}6$ in., Section 18.10.6.2 does require a boundary element.

Page 417, Step 5, 18.10.6.4(f), Column 2, revised as:

Table 18.10.6.4(f)(a):

$$0.3 \left(\frac{A_g}{A_{ch}} - 1 \right) \frac{f'_c}{f_{yt}}$$