



Standard Practice for Establishing Characteristic Values for Reinforced Glued Laminated Timber (Glulam) Beams Using Mechanics-Based Models¹

This standard is issued under the fixed designation D7199; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ϵ) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 This practice covers mechanics-based requirements for calculating characteristic values for the strength and stiffness of reinforced structural glued laminated timbers (glulam) manufactured in accordance with applicable provisions of ANSI/AITC A190.1, subjected to quasi-static loadings. It addresses methods to obtain bending properties parallel to grain, about the x-x axis (F_{bx} and E_x) for horizontally-laminated reinforced glulam beams. Secondary properties such as bending about the y-y axis (F_{by}), shear parallel to grain (F_{vx} and F_{vy}), tension parallel to grain (F_t), compression parallel to grain (F_c), and compression perpendicular to grain ($F_{c\perp}$) are beyond the scope of this practice. When determination of secondary properties is deemed necessary, testing according to other applicable methods, such as Test Methods [D143](#), [D198](#) or analysis in accordance with Practice [D3737](#), is required to establish these secondary properties. Reinforced glulam beams subjected to axial loads are outside the scope of this standard. This practice also provides minimum test requirements to validate the mechanics-based model.

1.2 The practice also describes a minimum set of performance-based durability test requirements for reinforced glulams, as specified in [Annex A1](#). Additional durability test requirements shall be considered in accordance with the specific end-use environment. [Appendix X1](#) provides an example of a mechanics-based methodology that satisfies the requirements set forth in this standard.

1.3 Characteristic strength and elastic properties obtained using this standard may be used as a basis for developing design values. However, the proper safety, serviceability and adjustment factors including duration of load, to be used in design are outside the scope of this standard.

1.4 This practice does not cover unbonded reinforcement, prestressed reinforcement, nor shear reinforcement.

1.5 The values stated in SI units are to be regarded as standard. The mechanics based model may be developed using SI or in.-lb units.

1.6 *This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.*

2. Referenced Documents

2.1 ASTM Standards:²

- [D9 Terminology Relating to Wood and Wood-Based Products](#)
- [D143 Test Methods for Small Clear Specimens of Timber](#)
- [D198 Test Methods of Static Tests of Lumber in Structural Sizes](#)
- [D905 Test Method for Strength Properties of Adhesive Bonds in Shear by Compression Loading](#)
- [D1990 Practice for Establishing Allowable Properties for Visually-Graded Dimension Lumber from In-Grade Tests of Full-Size Specimens](#)
- [D2559 Specification for Adhesives for Bonded Structural Wood Products for Use Under Exterior Exposure Conditions](#)
- [D2915 Practice for Sampling and Data-Analysis for Structural Wood and Wood-Based Products](#)
- [D3039/D3039M Test Method for Tensile Properties of Polymer Matrix Composite Materials](#)
- [D3410/D3410M Test Method for Compressive Properties of Polymer Matrix Composite Materials with Unsupported Gage Section by Shear Loading](#)
- [D3737 Practice for Establishing Allowable Properties for Structural Glued Laminated Timber \(Glulam\)](#)
- [D4761 Test Methods for Mechanical Properties of Lumber and Wood-Base Structural Material](#)
- [D5124 Practice for Testing and Use of a Random Number](#)

¹ This practice is under the jurisdiction of ASTM Committee [D07](#) on Wood and is the direct responsibility of Subcommittee [D07.02](#) on Lumber and Engineered Wood Products.

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² For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

Generator in Lumber and Wood Products Simulation

2.2 Other Standard:

ANSI/AITC A190.1 Structural Glued Laminated Timber³

3. Terminology

3.1 *Definitions*—Standard definitions of wood terms are given in Terminology D9 and standard definitions of structural glued laminated timber terms are given in Practice D3737.

3.2 Definitions of Terms Specific to This Standard:

3.2.1 *bonded reinforcement*—a reinforcing material that is continuously attached to a glulam beam through adhesive bonding.

3.2.2 *bumper lamination*—a wood lamination continuously bonded to the outer side of reinforcement.

3.2.3 *compression reinforcement*—reinforcement placed on the compression side of a flexural member.

3.2.4 *conventional wood lamstock*—solid sawn wood laminations with a net thickness of 2 in. or less, graded either visually or through mechanical means, finger-jointed and face-bonded to form a glulam.

3.2.5 *development length*—the length of the bond line along the axis of the beam required to develop the design tensile strength of the reinforcement.

3.2.6 *fiber-reinforced polymer (FRP)*—any material consisting of at least two distinct components: reinforcing fibers and a binder matrix (a polymer). The reinforcing fibers are permitted to be either synthetic (for example, glass), metallic, or natural (for example, wood), and are permitted to be long and continuously-oriented, or short and randomly oriented. The binder matrix is permitted to be either thermoplastic (for

example, polypropylene or nylon) or thermosetting (for example, epoxy or vinyl-ester).

3.2.7 *laminating effect*—an apparent increase of lumber lamination tensile strength because it is bonded to adjacent laminations within a glulam beam. This apparent increase may be attributed to a redirection of stresses around knots and grain deviations through adjacent laminations.

3.2.8 *partial length reinforcement*—reinforcement that is terminated within the length of the timber.

3.2.9 *reinforcement*—any material that is not a conventional lamstock whose mean longitudinal ultimate strength exceeds 20 ksi for tension and compression, and whose mean tension and compression MOE exceeds 3000 ksi, when placed into a glulam timber. Acceptable reinforcing materials include but are not restricted to: fiber-reinforced polymer (FRP) plates and bars, metallic plates and bars, FRP-reinforced laminated veneer lumber (LVL), FRP-reinforced parallel strand lumber (PSL).

3.2.10 *shear reinforcement*—reinforcement intended to increase the shear strength of the beam. This standard does not cover shear reinforcement.

3.2.11 *tension reinforcement*—reinforcement placed on the tension side of a flexural member.

3.3 Symbols:

M = moment arm, distance between compression and tension force couple applied to beam cross-section

b = beam width

C = total internal compression force within the beam cross-section (see Fig. 2)

CFRP = carbon fiber reinforced polymer

d = beam depth

E = long-span flatwise-bending modulus of elasticity for wood lamstock (Test Methods D4761; also see Fig. 1)

F_b = allowable bending stress parallel to grain

³ Available from American National Standards Institute (ANSI), 25 W. 43rd St., 4th Floor, New York, NY 10036, <http://www.ansi.org>.

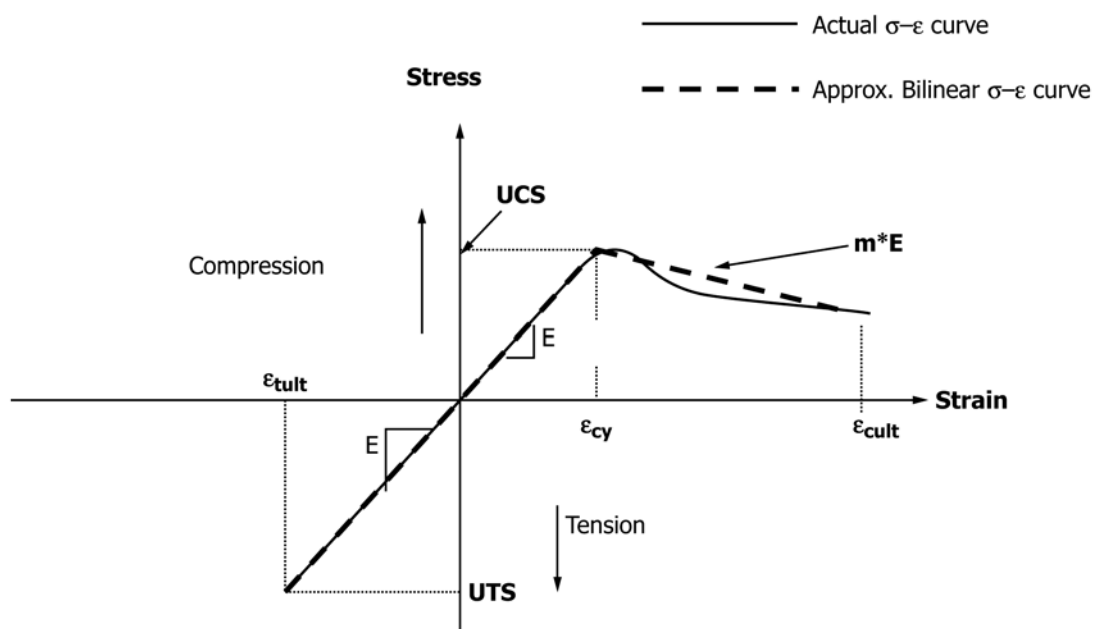
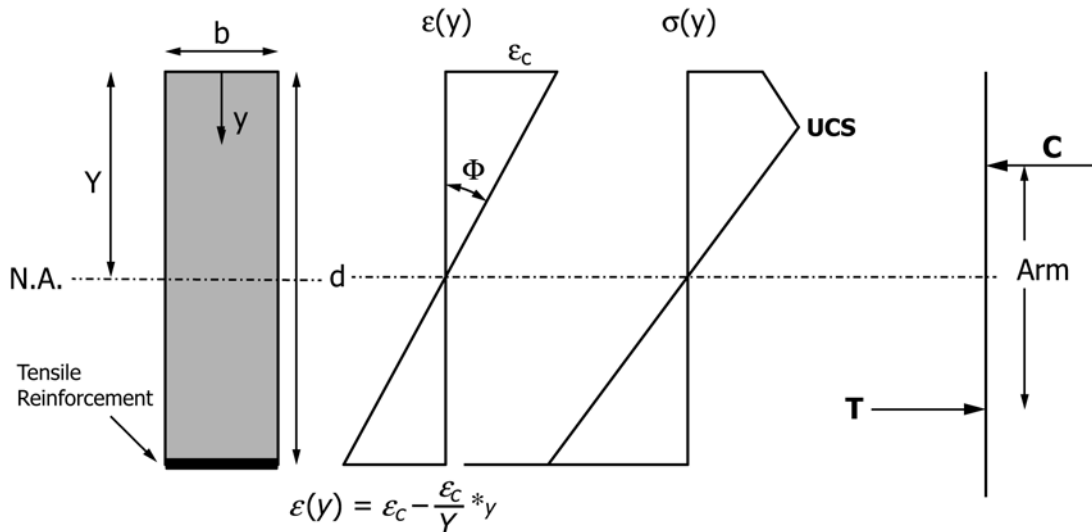


FIG. 1 Typical Stress-Strain Relationship for Wood Lamstock, with Bilinear Approximation



NOTE 1—A simplified rectangular block stress distribution can be used but it must be shown that it accurately represents the stress distribution.

FIG. 2 Example of Beam Section with Strain, Stress, and Force Diagrams

F_x = internal horizontal force on the beam cross-section (see Eq 2)

GFRP = Glass fiber-reinforced polymer

LEL = lower exclusion limit (point estimate with 50 % confidence, includes volume factor)

LTL = lower tolerance limit (typically calculated with 75 % confidence)

$M_{applied}$ = external moment applied to the beam cross-section

$M_{internal}$ = internal moment on the beam cross-section

MC = moisture content (%)

MOE = modulus of elasticity

MOR = modulus of rupture

$MOR_{5\%}$ = 5 % one-sided lower tolerance limit for modulus of rupture, including the volume factor

$MOR_{BL5\%}$ = 5 % one-sided lower tolerance limit for modulus of rupture corresponding to failure of the bumper lamination, including the volume factor

$m * E$ = downward slope of bilinear compression stress-strain curve for wood lamstock (see Fig. 1)

N.A. = neutral axis

T = total internal tension force within the beam cross-section (see Fig. 2)

UCS = ultimate compressive stress parallel to grain

UTS = ultimate tensile stress parallel to grain

Y = distance from extreme compression fiber to neutral axis (see Fig. 2)

y = distance from extreme compression fiber to point of interest on beam cross-section (see Fig. 2)

ϵ_c = strain at extreme compression fiber of beam cross-section (see Fig. 2)

ϵ_{cult} = compression strain at lamstock failure (see Fig. 1)

ϵ_{cy} = compression yield strain at lamstock UCS (see Fig. 1)

ϵ_{ult} = tensile strain at lamstock failure (see Fig. 1)

$\epsilon(y)$ = strain distribution through beam depth (see Fig. 2)

ρ = tension reinforcement ratio (%); cross-sectional area of tension reinforcement divided by cross-sectional area of beam between the c.g. of tension reinforcement and the extreme compression fiber

ρ' = compression reinforcement ratio (%); cross-sectional area of compression reinforcement divided by cross-sectional area of beam between the c.g. of compression reinforcement and the extreme tension fiber

$\sigma(y)$ = stress distribution through beam depth (see Fig. 2)

4. Requirements for Mechanics-Based Analysis Methodology

NOTE 1—At a minimum, the mechanics-based analysis shall account for: (1) Stress-strain relationships for wood laminations and reinforcement; (2) Strain compatibility; (3) Equilibrium; (4) Variability of mechanical properties; (5) Volume effects; (6) Finger-joint effects; (7) Laminating effects; and (8) Stress concentrations at termination of reinforcement in beams with partial length reinforcement. In addition to the above factors, characteristic values developed using the mechanics-based model need to be further adjusted to address end-use conditions including moisture effects, duration of load, preservative treatment, temperature, fire, and environmental effects. The development and application of these additional factors are outside the scope of this practice. Annex A1 addresses the evaluation of durability effects. The minimum output requirements for the analysis are mean MOE (based on gross section) and 5% LTL MOR with 75 % confidence (based on gross section), both at 12 % MC. These analysis requirements are described below.

4.1 Stress-strain Relationships:

4.1.1 Conventional Wood Lamstock:

4.1.1.1 The stress-strain relationship shall be established through in-grade testing following Test Methods D198 or Test Methods D4761, or other established relationships as long as the resulting model meets the criteria established in Section 5. Test lamstock shall be sampled in sufficient quantity from enough sources to insure that the test results are representative of the lamstock population that will be used in the fabrication of the beams. Follow-up testing shall be performed to ensure

order to track changes in lamstock properties over time, so that the layup designs may be adjusted accordingly.

4.1.1.2 The stress-strain relationship shall be linear in tension. The stress-strain relationship shall be nonlinear in compression if compression is the governing failure mode. In this case, a bilinear approximation is acceptable, and shall be used throughout this standard (see Fig. 1). In the bilinear model both tension and compression MOE shall be permitted to be approximated by using the long-span flatwise-bending MOE obtained using Test Methods D4761. In Fig. 1, m^*E is the downward slope of the compression stress-strain curve, defined as the best-fit downward line through the point (UCS, ϵ_{cy}) on the compression stress-strain curve. The downward best-fit line shall be permitted to be terminated at the point where the ultimate compressive strain ϵ_{cu} is approximately 1 %.

4.1.2 Reinforcement:

4.1.2.1 The stress-strain relationship shall be established through material-level testing in accordance with Test Method D3039/D3039M and D3410/D3410M.

4.1.2.2 Nonlinearities in the stress-strain relationship shall be included in the analysis, if present.

4.1.2.3 Acceptable stress-strain models for unidirectional E-glass FRP (GFRP), Aramid, or Carbon FRP (CFRP) in tension are linear-elastic. Acceptable models for hybrid E-glass/Carbon composites in tension are linear or bilinear. Acceptable models for mild steel reinforcement are elastic-plastic. Similar models may also apply in compression.

4.2 Strain Compatibility:

4.2.1 Fig. 2 shows the cross section of a beam with a linear strain and bilinear stress distribution, with the neutral axis a distance Y below the top of the beam. Using the extreme compression fiber as the origin, the strain distribution for a given applied moment (M_{applied}) is defined by the equation:

$$\epsilon(y) = \epsilon_c - \epsilon_c^*(y/Y) \quad (1)$$

4.3 Equilibrium:

4.3.1 In order to maintain equilibrium, the cross-section shall satisfy the conditions of horizontal equilibrium (Eq 2), and the internal moment (M_{internal}) shall equal the external moment applied to that cross section (M_{applied}) (Eq 3). See Fig. 2 as an example of strain compatibility and equilibrium:

$$\sum F_x = 0 \Rightarrow \int_{\text{depth}} \sigma(y) dA = 0 \quad (2)$$

$$M_{\text{applied}} = M_{\text{internal}} = C(\text{or } T) * \text{Arm} = \int_{\text{depth}} -y * \sigma(y) * dA \quad (3)$$

4.4 Variability of Mechanical Properties:

4.4.1 The model shall properly account for the variability of the mechanical properties of the wood lamstock and the FRP reinforcement. This includes variability of individual properties and correlations among those properties as appropriate. The mechanics-based analysis shall address statistical properties for and correlations between Ultimate Tensile Stress (UTS), Ultimate Compressive Stress (UCS) and long-span flatwise-bending modulus of elasticity (E). One example of how this may be achieved is provided in Appendix X1.

4.4.2 These correlation values are obtained from test data. Test lamstock shall be sampled in sufficient quantity, from enough sources to insure that the test results are representative

of the lamstock population that will be used in the fabrication of the beams. Follow-up testing shall be performed annually in order to track changes in lamstock properties over time, so that the layup designs may be adjusted accordingly.

4.5 Volume Effects:

4.5.1 The model shall properly account for changes in beam strength properties as affected by beam size. In conventional glulam, this is achieved by using a volume factor C_v , which was derived from laboratory test data. With adequate reinforcement, glulams can achieve a reduction or even elimination of volume effects. The model shall properly account for this phenomenon. One possible approach to address the volume effect is described in Appendix X1.

4.6 Finger-Joint Effects:

4.6.1 Finger joints affect the mechanical properties of lamstock used in glulams. The model shall account for these effects on both the mean and variability of the beam mechanical properties. One example of how this may be achieved is provided in Appendix X1.

4.7 Laminating Effects:

4.7.1 The laminating effects may be predicted by the model or else developed outside the model (and applied in the model) using an empirical, numerical or analytical approach. One way to achieve this for a beam subjected to 4-point bending is described in Appendix X1.

4.8 Stress Concentrations at Termination of Reinforcement in Beams with Partial Length Reinforcement:

4.8.1 Beams with partial length reinforcement have stress concentrations near the ends of the reinforcement. These stress concentrations are in the form of tension or compression stresses parallel to grain, combined with peeling stresses perpendicular to grain. The model shall have the ability to account for the effects of these stress concentrations if partial length reinforcement will be used.

4.9 Mechanical Properties Predicted by Model:

4.9.1 The model shall at a minimum predict the following properties, including the effects of a bumper lamination if one is used, which are the basis for design values.

4.9.2 Bending Strength:

4.9.2.1 The bending strength calculated by the model assumes adequate bond development length is provided for the reinforcement. The model shall predict the lower 5 % tolerance limit for modulus of rupture ($MOR_{5\%}$) for the reinforced layup being analyzed. Beam MOR shall be based on gross (full width and depth) cross section properties:

$$MOR = \frac{6 * M_{\text{max}}}{b * d^2} \quad (4)$$

Where M_{max} is the maximum moment applied to the beam, and b and d are respectively the full width and depth of the beam cross-section. The transformed section properties shall not be used.

4.9.2.2 If a bumper lamination is used, an additional characteristic bending strength value $MOR_{\text{BLS}\%}$ corresponding to bumper lamination failure shall also be reported. It should be noted that the model-predicted bending strength shall not be used